

Parklea Correctional Centre Expansion Stormwater Management Report

20 July 2016 | 15-207

Contents

Contents	2
Document control	3
1. Introduction	4
2. Existing Site	5
3. Council requirements	7
4. Proposed Development	8
5. Stormwater Management	8
6. Flooding	11
Appendix A - Drawings	12
Appendix B - Calculations	13

Document control

Rev No	Date	Revision details	Approved	Verified	Prepared
A	20/7/16	Approved Issue	CMW	SETB	JG

Copyright 2016 © Woolacotts Consulting Engineers | Do not use, copy, or reproduce wholly or in part without written permission

1. Introduction

Parklea Correctional Centre is located off Sentry Drive, Parklea. An expansion of the centre is proposed, to provide additional accommodation on site, with the proposed works including:

- 500 maximum security beds within the secure perimeter wall
- 150 minimum security beds outside the secure perimeter wall
- Minimum security visits building
- Minimum security support services building
- Program/ Industries building
- AVL/ Clinic building
- 2 movement control post buildings
- Playing fields and recreational areas
- Secure covered walkways
- Additional visitor carpark and associated roadworks.

This report has been prepared to document stormwater management issues associated with the proposed works. It will address existing conditions at the site, details of the proposed works, Blacktown City Council's requirements, the measures proposed for the site and the means of compliance with Council's requirements.

2. Existing Site

The site is located off Sentry Drive, Parklea. Refer to Figure 1 for the site location.

The existing correctional centre is comprised of buildings, parking areas and roads. Two dams are located on the western side of the site. Refer to Figure 2 for an aerial photograph of the site.

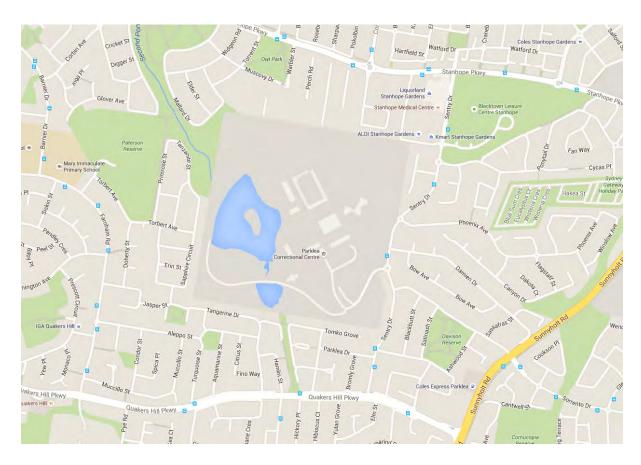


Figure 1 – Site Location



Figure 2 – Aerial photograph of the site (Photograph © Six Maps.)

3. Council requirements

Blacktown City Council provided input to the draft Secretary's Environmental Assessment Requirements for the proposed development. For stormwater drainage from the site, these requirements can be summarised as follows:

Stormwater quality

Stormwater quality improvement targets are to be achieved for the new prison development only. The targets are to be assess using MUSIC in accordance with Council's water sensitive urban design information sheets and developer handbook. For the proposed works, the required percentage reduction in post development average annual pollutant loads are:

•	Gross pollutants	90%
•	Total suspended solids	85%
•	Total phosphorous	65%
•	Total Nitrogen	45%
	Total hydrocarbons	90%

As MUSIC does not assess hydrocarbons, Council requires that a gross pollutant trap targeting hydrocarbons be provided for the new carpark and roadways, with the device designed to treat the 6 month average recurrence interval (ARI) flow.

Rainwater harvesting

Council requires that the development achieves a minimum of 80% of the non-potable water uses onsite for the new prison development being met using rainwater. Non-potable uses include toilet flushing and landscape watering. The usage rate is to be assessed using MUSIC.

In undertaking the assessment, the following usage is to be adopted:

•	Communal toilet use	0.1 kL/day per toilet or urinal
•	Single cell	0.025 kL/ day
•	Double cell	0.05 kL/day
ě.	Watering of landscaped areas	0.4 kL/year/m ² . (as PET-Rain)

In addition, for modelling purposes, a 20% reduction in tank size is to be used in MUSIC, to allow for losses in the tank.

Stormwater flows

An on-site detention system is required to mitigate post development flows from the new development, so that they do not exceed the peak pre-development flows for a range of storm events, ranging from the 1 in 2 year ARI to the 1 in 100 year ARI.

Council requires that the detention system design be supported using a computer model that complies with their *Engineering Requirements for Development*.

We note that Parklea is located in an area that is noted as exempt form OSD due to an area wide basin in the *Engineering Requirements for Development*.

4. Proposed Development

The proposed development consists of a number of new minimum and maximum security buildings. The new parking area is located off Sentry Drive. The new minimum security buildings are located to the south east of the existing complex, and the new maximum security facilities are located to the north east and north west, adjacent to the existing maximum security buildings.

The new buildings and associated works cover an area of 107,037 m², of which 61,758 m² is impervious. The parking areas have a total of area of 6,263 m².

5. Stormwater Management

A stormwater management system has been developed to accommodate the redevelopment work resulting in the increased impervious areas, as well as comply with Council's requirements.

The piped stormwater drainage system is designed to carry runoff from storms up to and including the 20 year ARI event, with pipes graded at a minimum fall of 1 in 100. A DRAINS model of the site has also been developed as part of the analysis.

For runoff from storms up to and including the 100 year ARI event, overland flow paths ensure a minimum freeboard to habitable levels. External surfaces are also graded at a minimum fall of 1 in 100 to the stormwater collection and drainage system. Refer to Appendix A – Drawings, for additional detail.

Stormwater detention

Council requires that an on-site detention system be provided for the proposed works only, and not the full site. The works proposed can be divided into two areas – the new parking area and the new buildings.

Stormwater from the site currently discharges to the existing detention basin at the south west of the site. The minimum security facility discharges to the south basin, and the maximum security discharges to the larger northern basin. Stormwater is temporarily stored at these locations and discharges into Second Ponds Creek.

As part of the proposed stormwater system analysis, the capacity of the existing basins were investigated. Computer modelling program XP_RAFTS was used to determine the peak outlet flow rates of multiple storm events of the pre-developed site. Refer to Appendix B – Calculations, for additional detail.

Catchment areas of the north and south basins, in conjunction with the existing dimensions of the basins were investigated in order to determine a baseline result. Following determination of the predeveloped peak flow rate within the model, a post-developed model was created to investigate the effects of the increase in impervious area, as well as identify the compatibility of the existing basins.

The increase in impervious area of the site resulted in an increase of peak flow rate from the basins. In order to satisfy the Secretary's Environmental Assessment Requirements, an increase of detention volume was necessary. The northern basin was discovered to be capable of storing additional volume with minor modification. A final model was developed to investigate the impacts of raising the existing spillway by 50mm. Refer to Appendix A – Drawings, for additional Detail. This modification resulted in an overall increase of available detention volume, and having a direct reduction on the discharged peak flowrates to below the pre-developed levels of the total site.

The spillway modification of the northern basin has satisfied that no detention in the upstream area will be required.

Water quality

Water quality improvement measures are proposed for the development, and consist of:

- Enviropods
- Grassed swales
- Rainwater reuse tanks
- Existing detention basins
- Gross Pollutant Trap

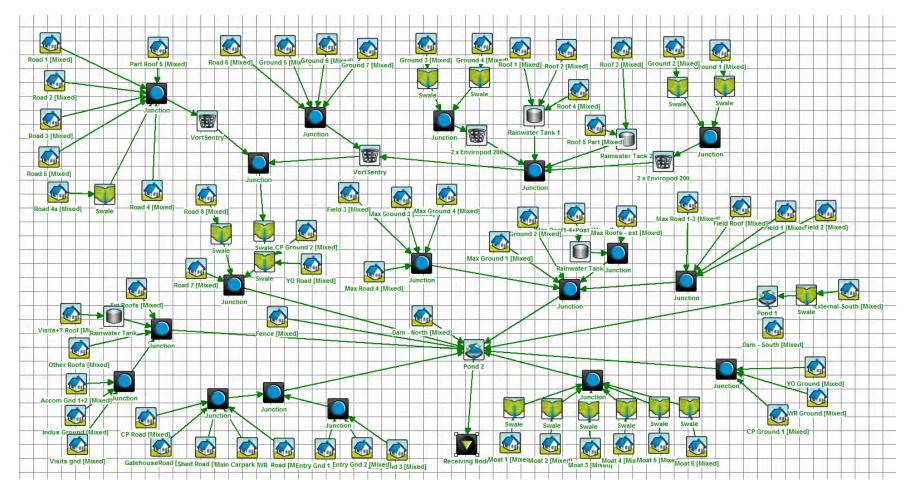
Refer to Appendix A – Drawings, for additional Detail.

As required by Council, the measures were modelled using MUSIC. To ensure that Council's modelling requirements are met, the model was set up using the standard Blacktown Council model, available through MUSIC-Link, with standard nodes available through MUSIC-Link used. Refer to Appendix B – Calculations, for additional detail.

A combination of the above listed control devices have been implemented to improve the overall water quality as a result of the proposed works. The following is a summary of the post-developed values satisfying Council's reduction in annual average pollutant loads.

•	Gross pollutants	100%
٠	Total suspended solids	95.8%
٠	Total phosphorous	93.2%
٠	Total Nitrogen	89%
÷	Total hydrocarbons	Vort Sentry GPT (targeting hydrocarbons)

Refer to Figure 3 below for a summary of the MUSIC treatment train.



× Treatment Train Effectiveness - Receiving Node Residual Load % Reduction Sources Flow (ML/yr) 441 55.1 87.5 Total Suspended Solids (kg/yr) 2800 95.8 66100 Total Phosphorus (kg/yr) 124 8.45 93.2 Total Nitrogen (kg/yr) 922 102 89 Gross Pollutants (kg/yr) 10300 0 100

Figure 3 – MUSIC Treatment Train

Erosion and sediment control

Erosion and sediment control measures area required for the site. It is proposed to provide measures in accordance with the "Blue Book" – *Managing Urban Stormwater* – *Soils and Construction 4th Edition*. The measures will include:

- Silt fences on the low side of the site
- Diversion swales upstream of the works
- Silt traps at existing pits near works

Refer to Appendix A – Drawings, for additional details.

6. Flooding

From Council's Flood Map, the site is not in a flood affected area. Therefore no additional modifications are required. Refer to Figure 4 below for additional information.

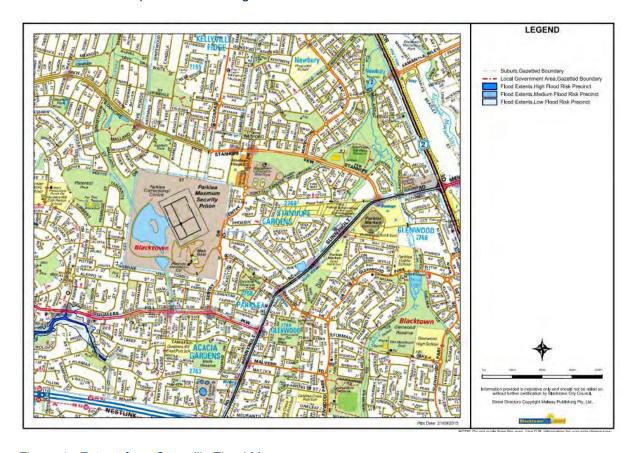
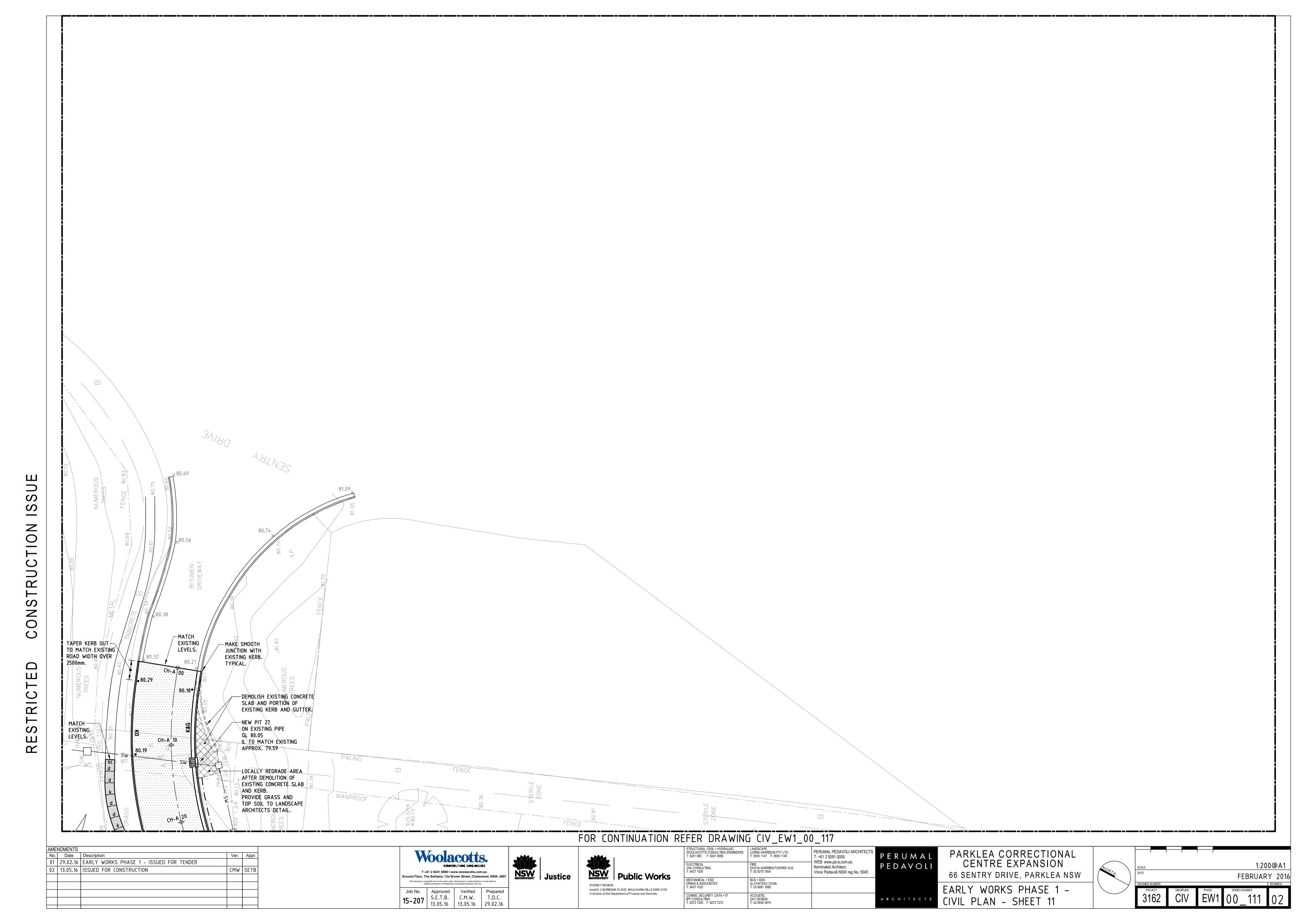
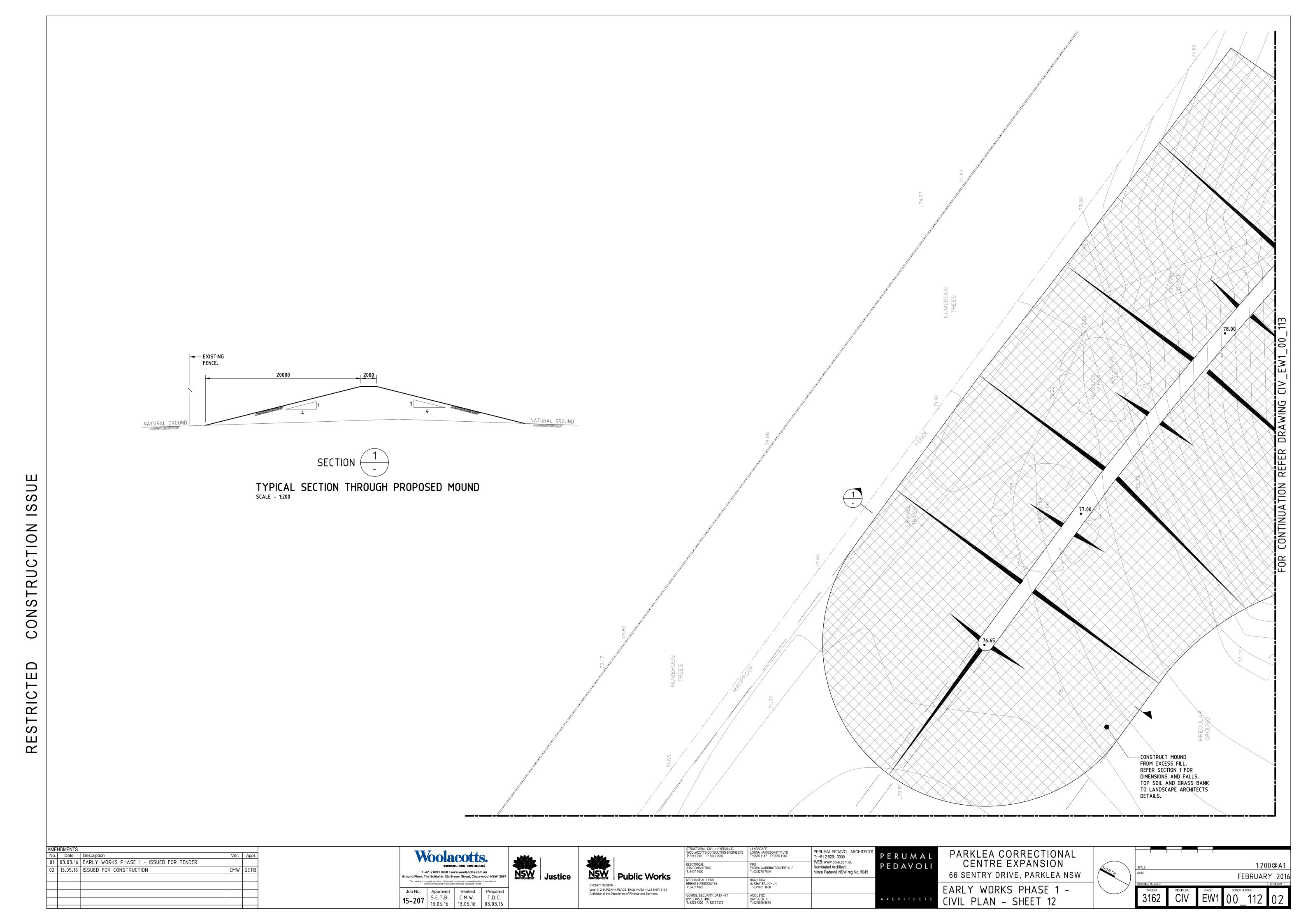
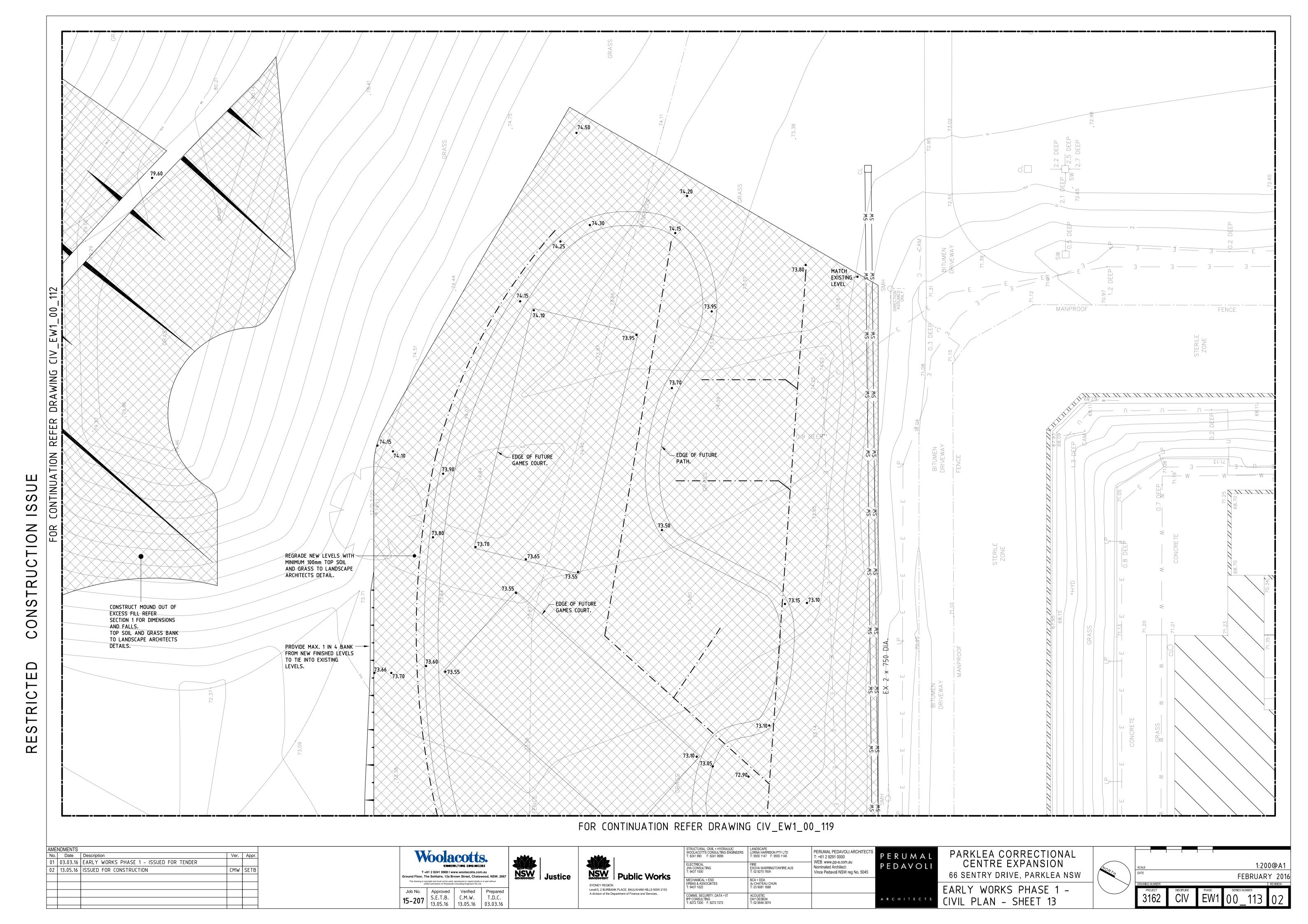


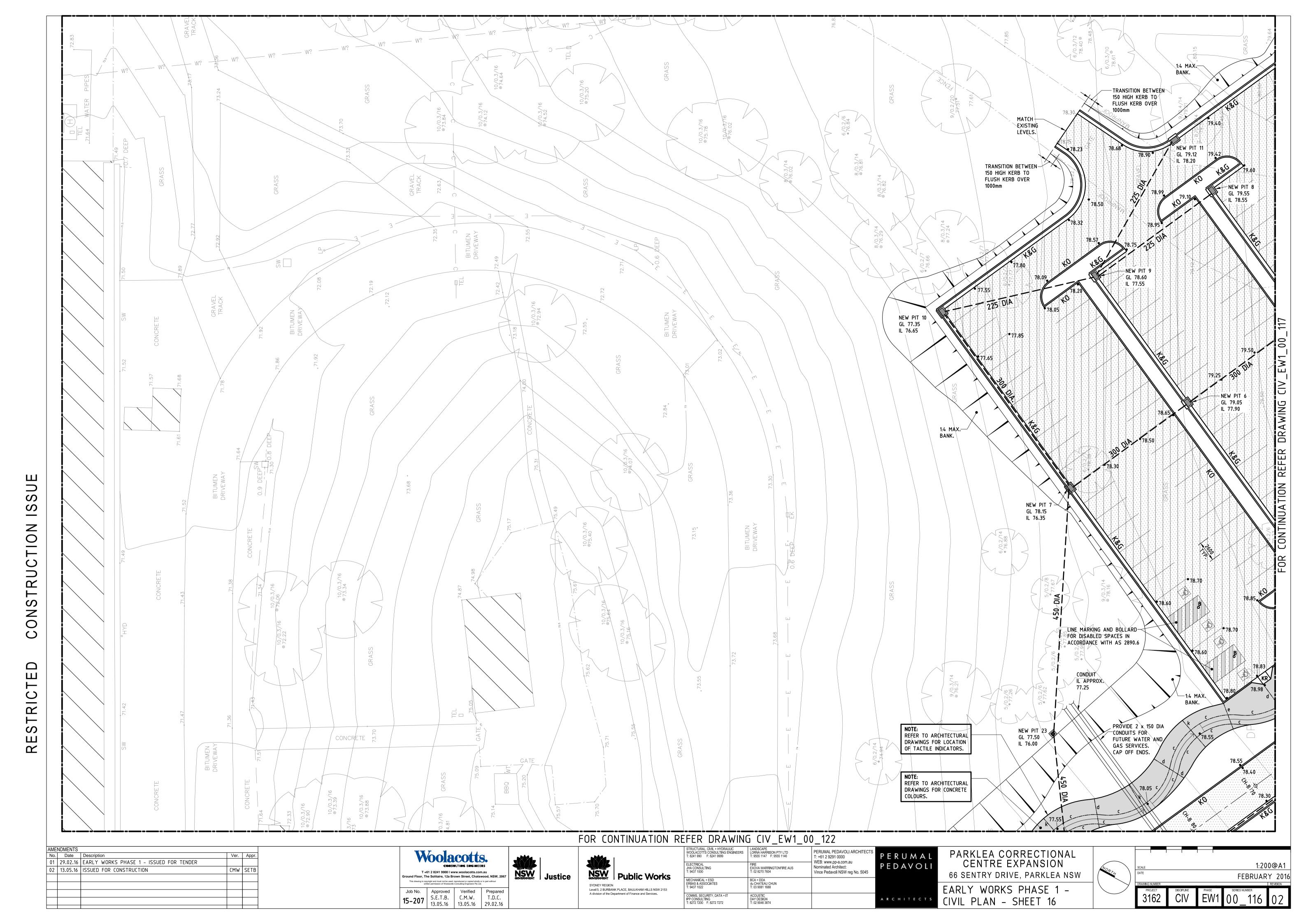
Figure 4 - Extract from Council's Flood Map

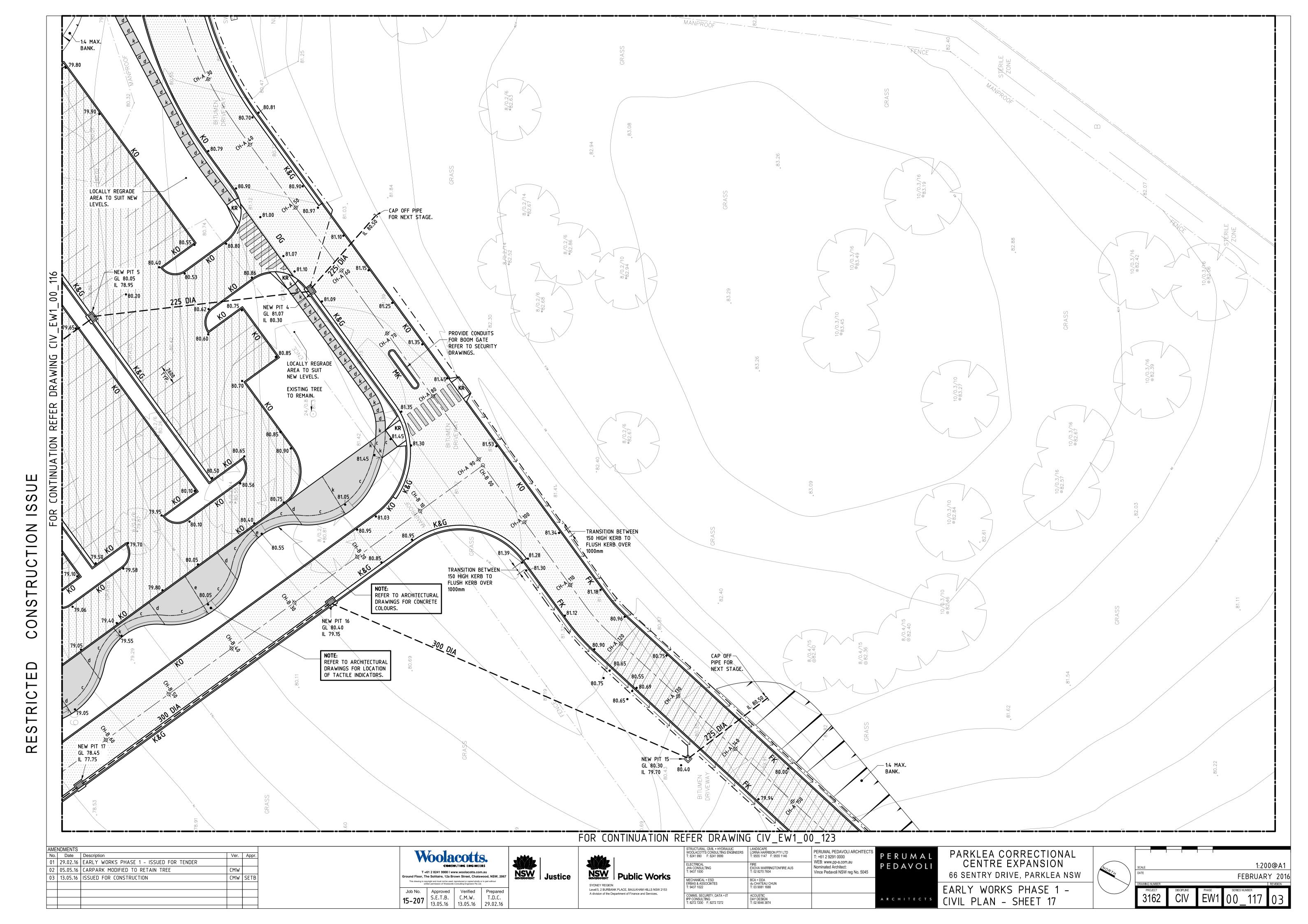
Appendix A Drawings

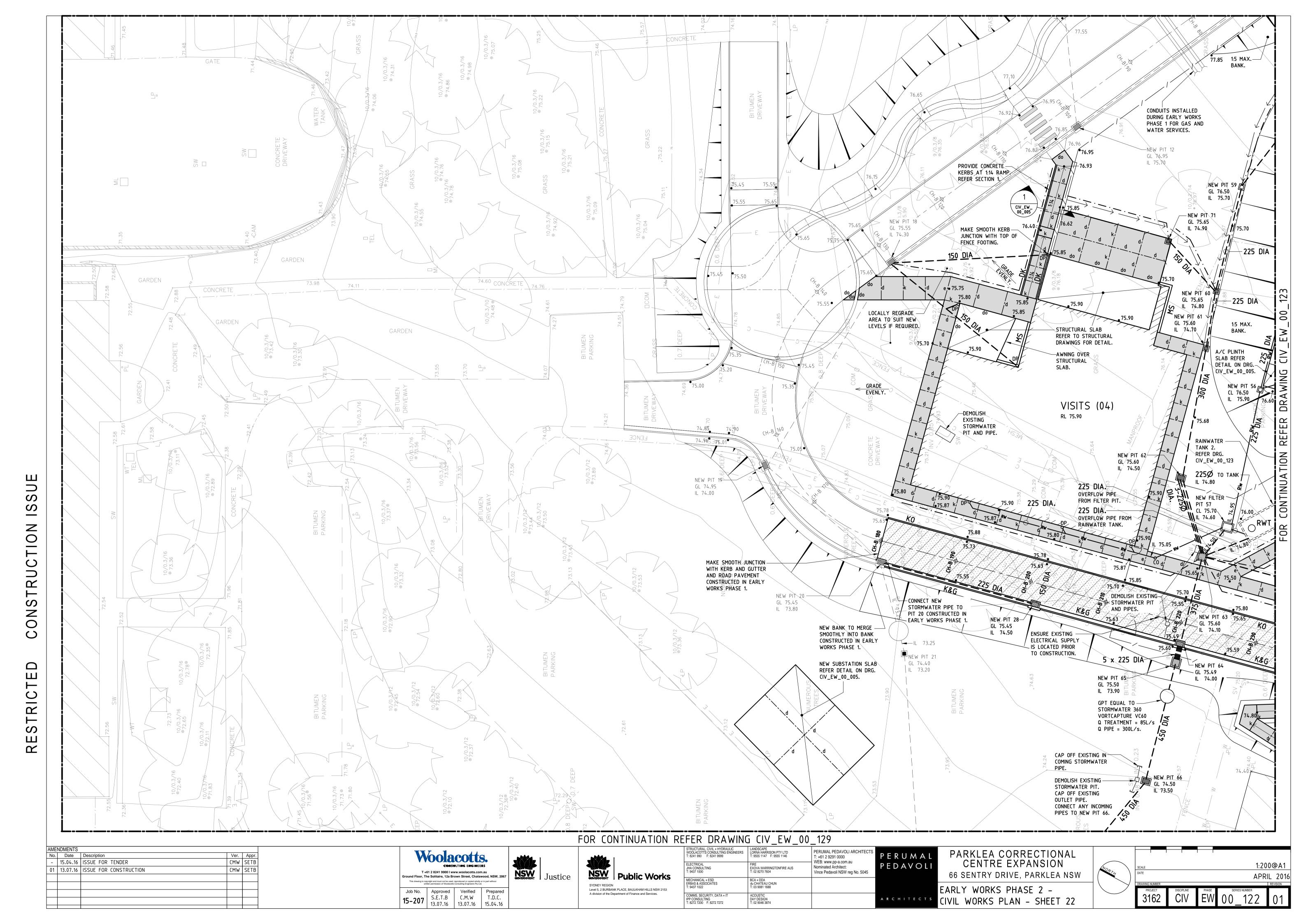


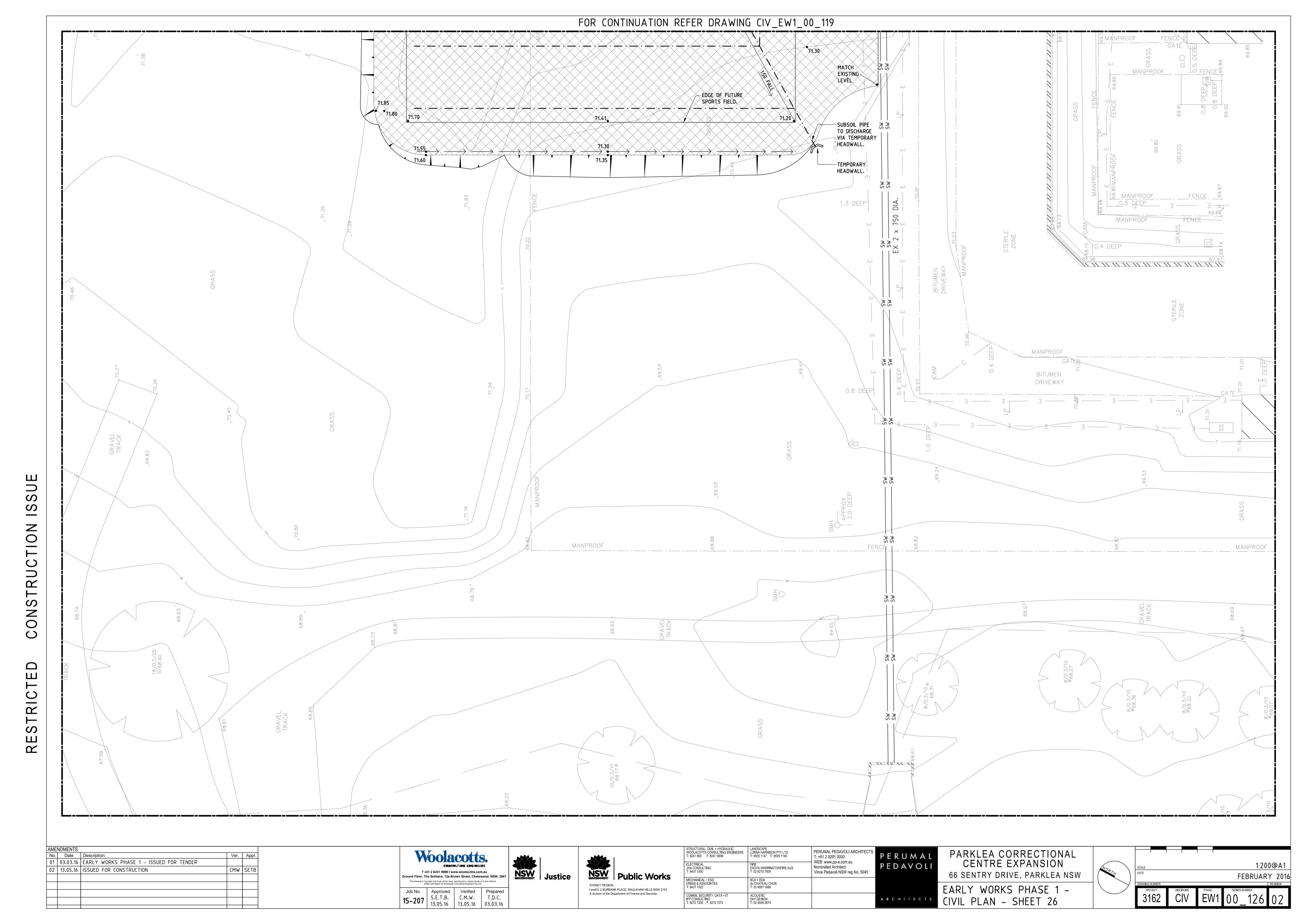


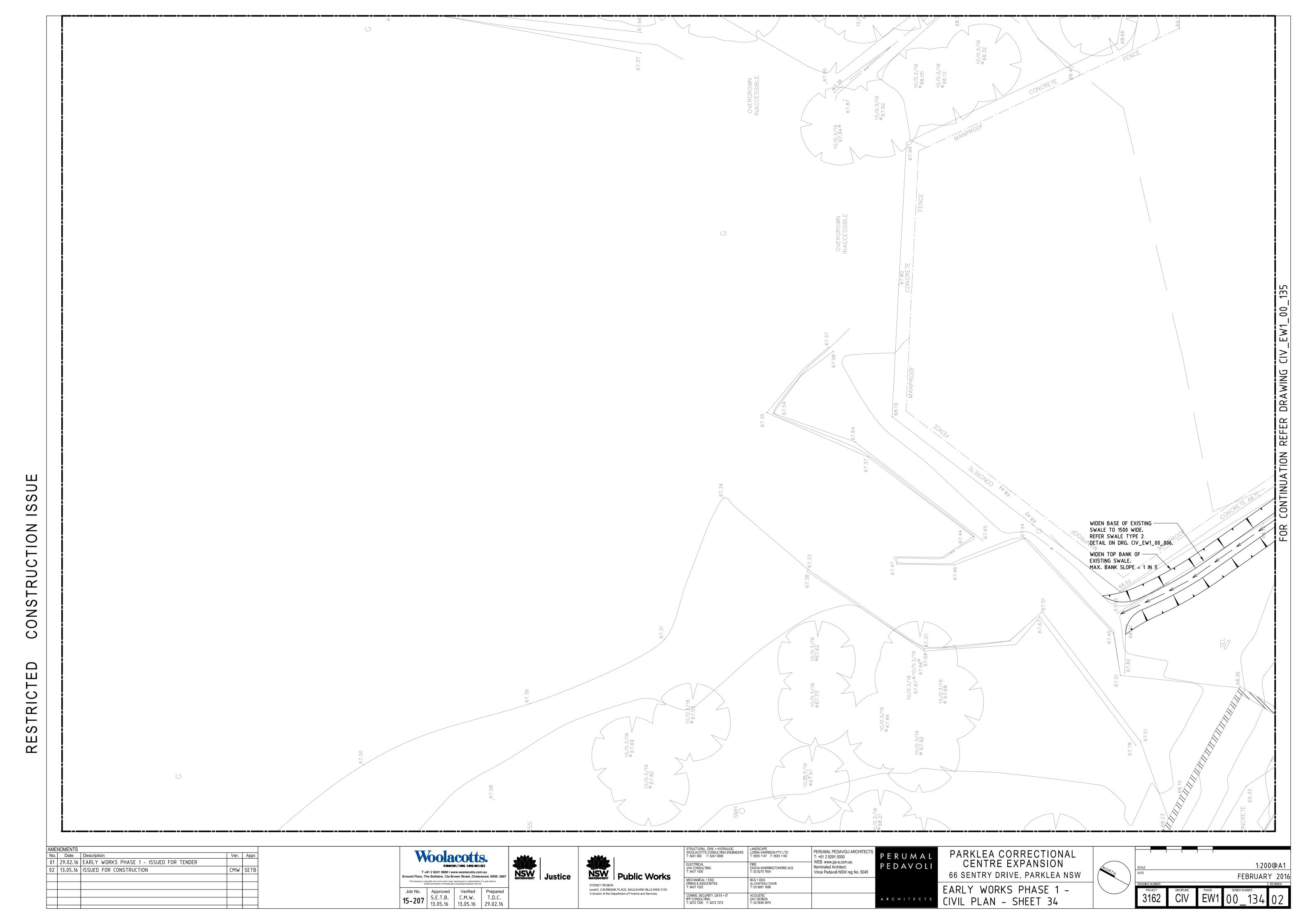


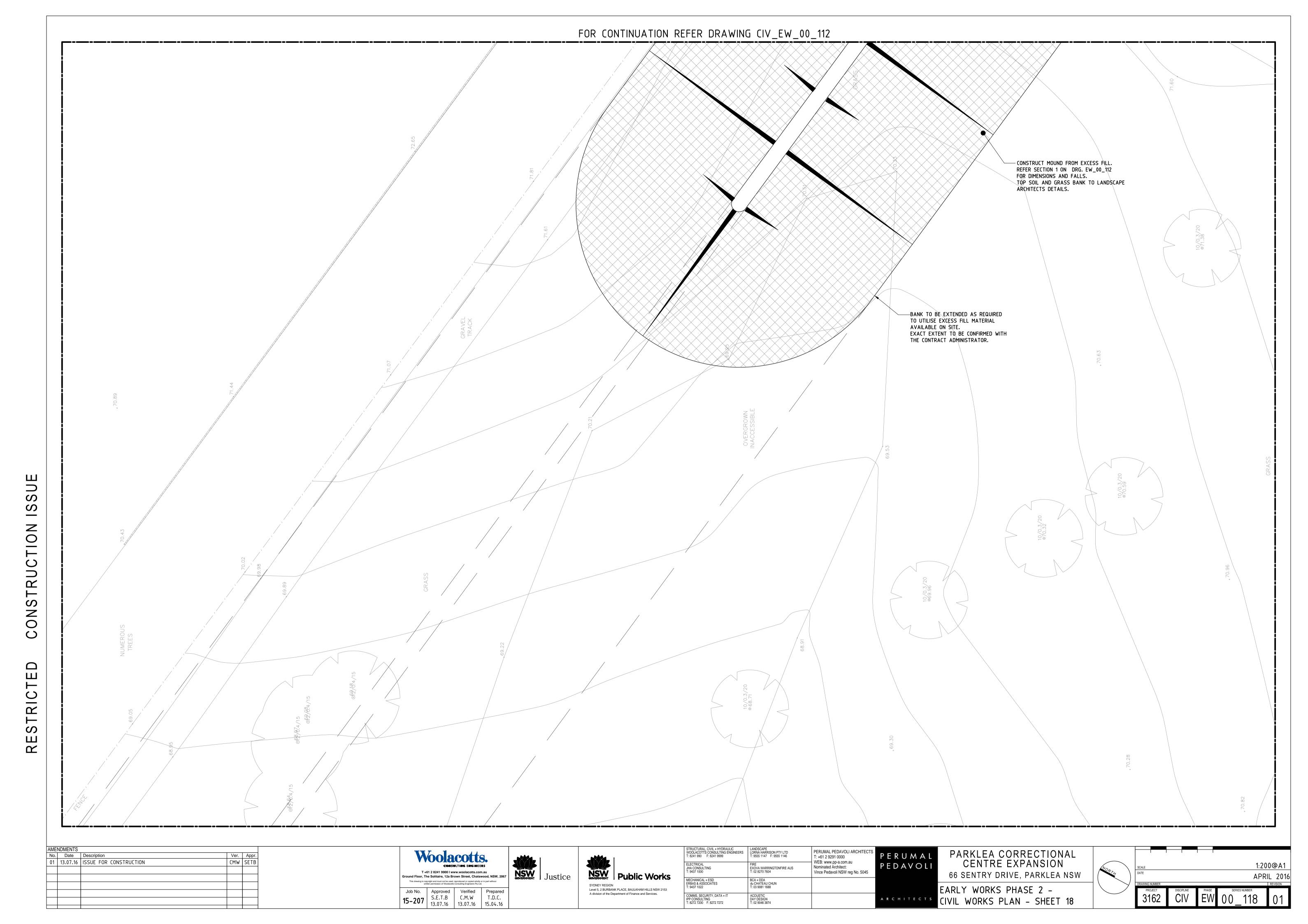


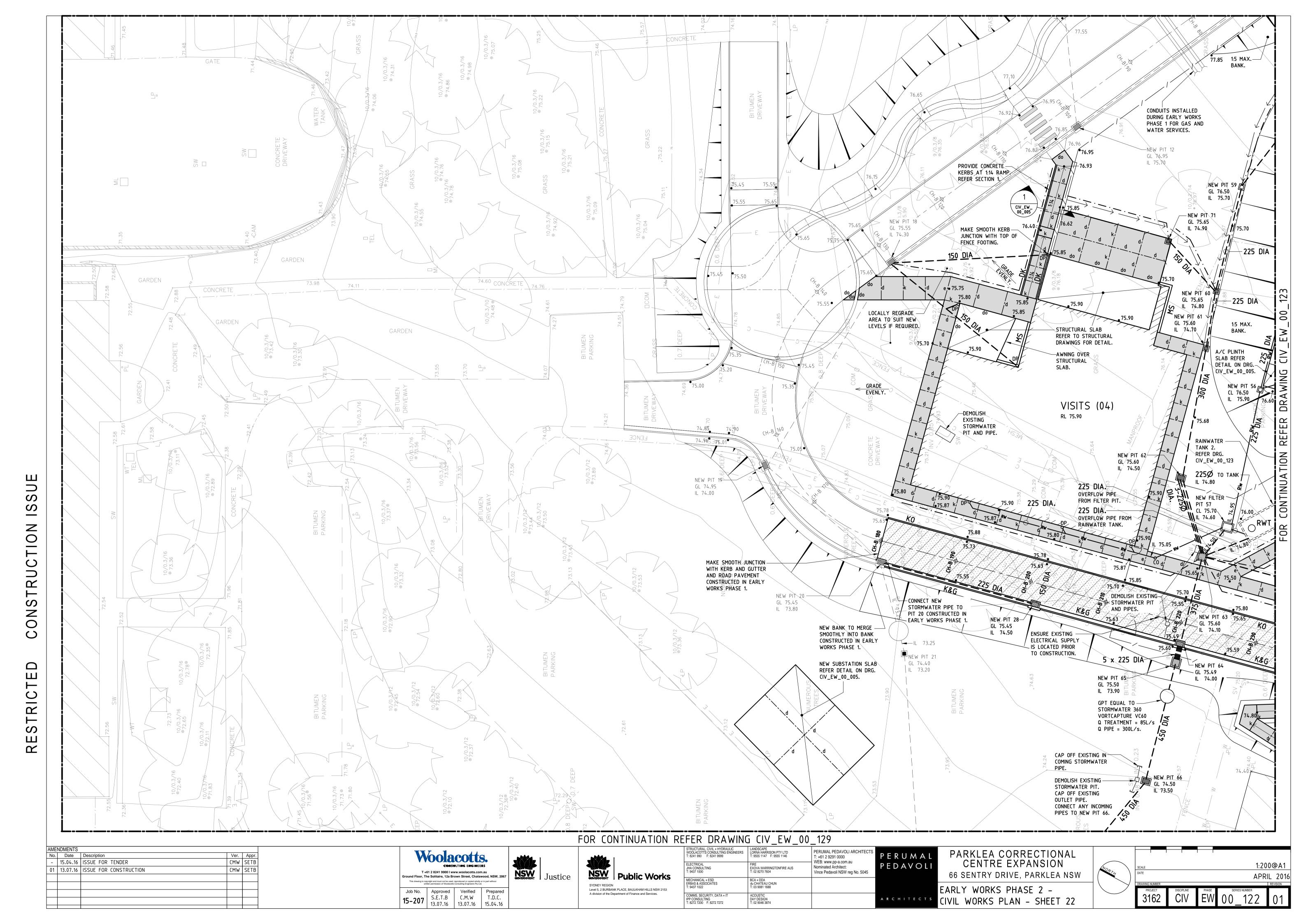


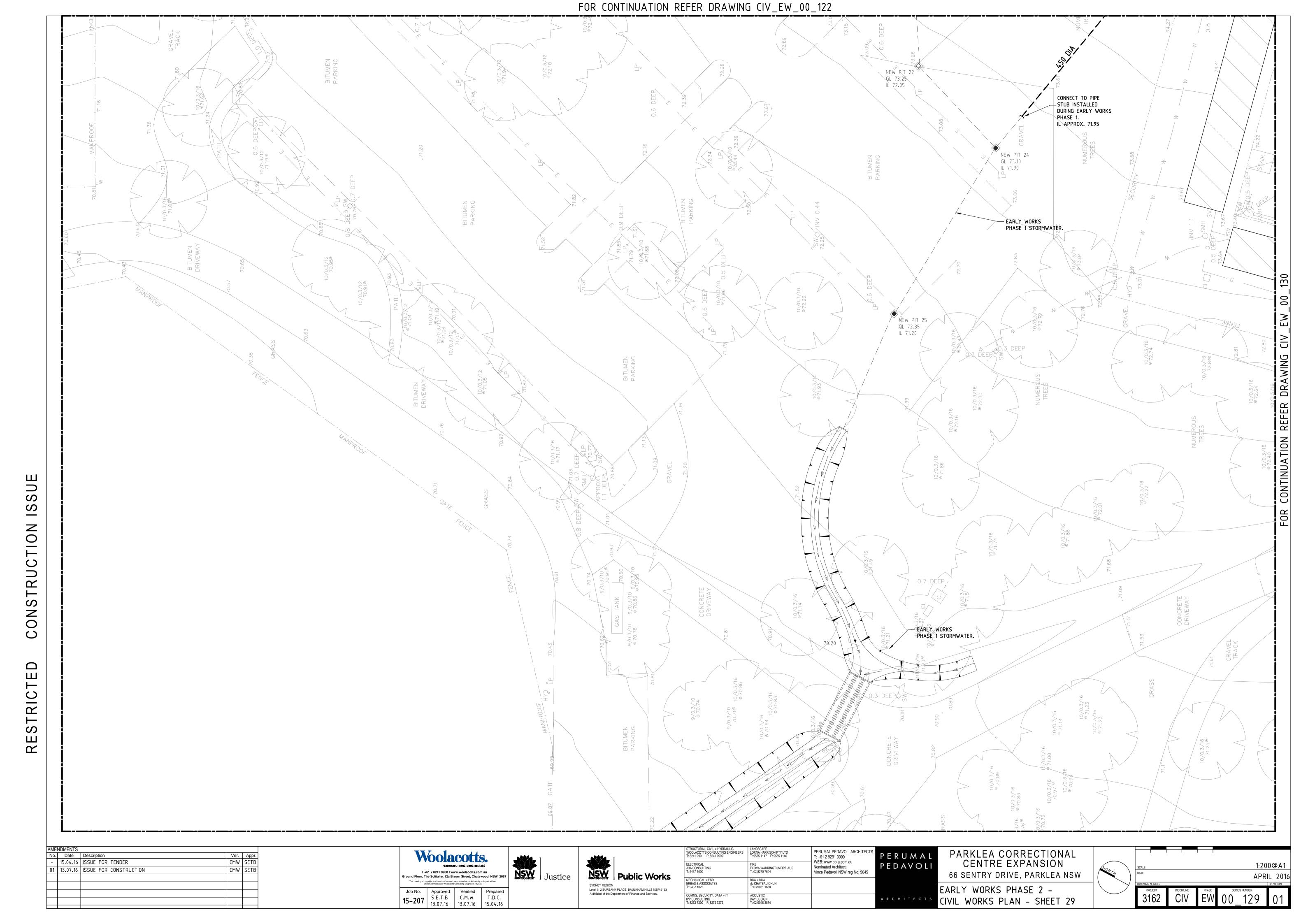


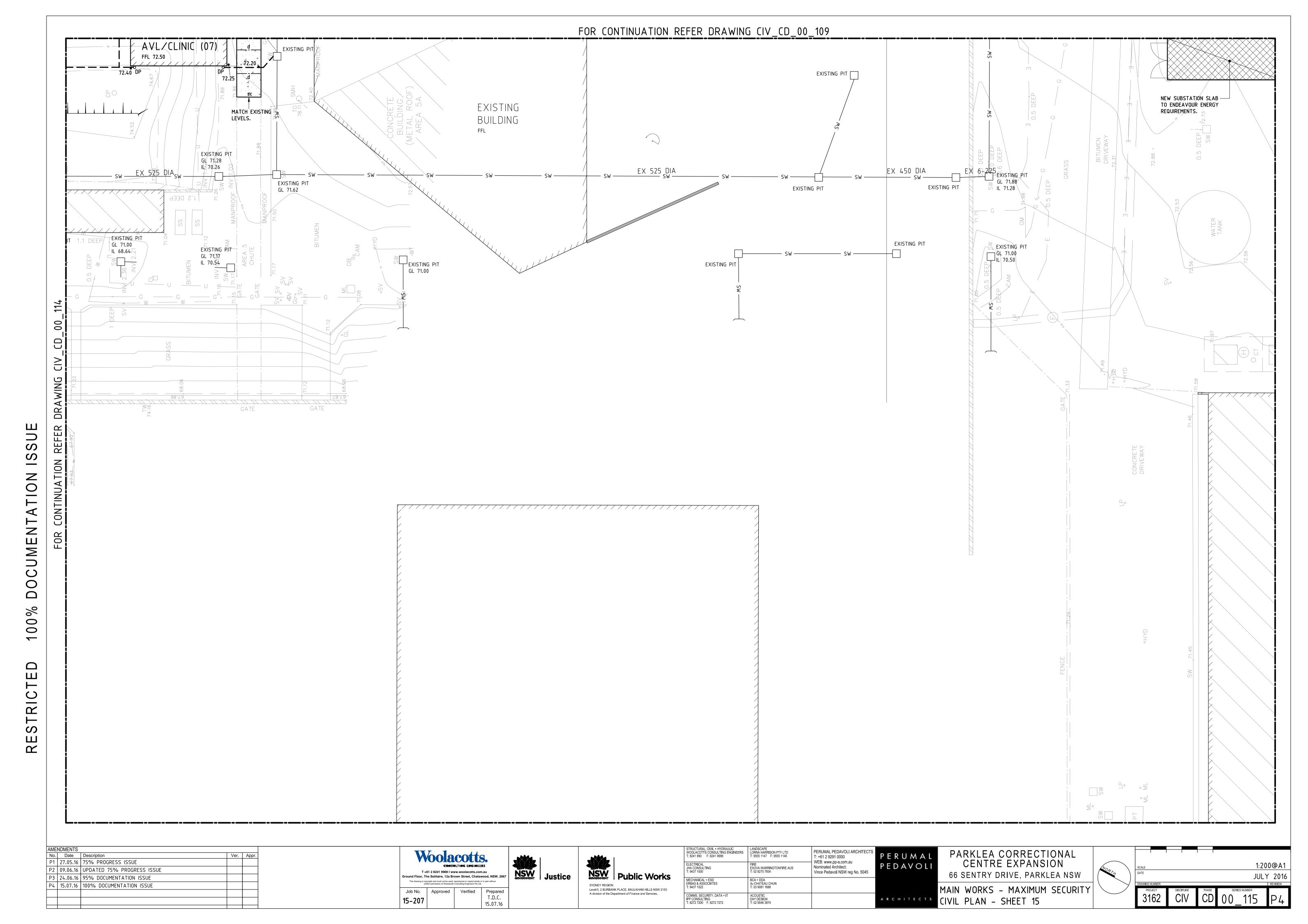


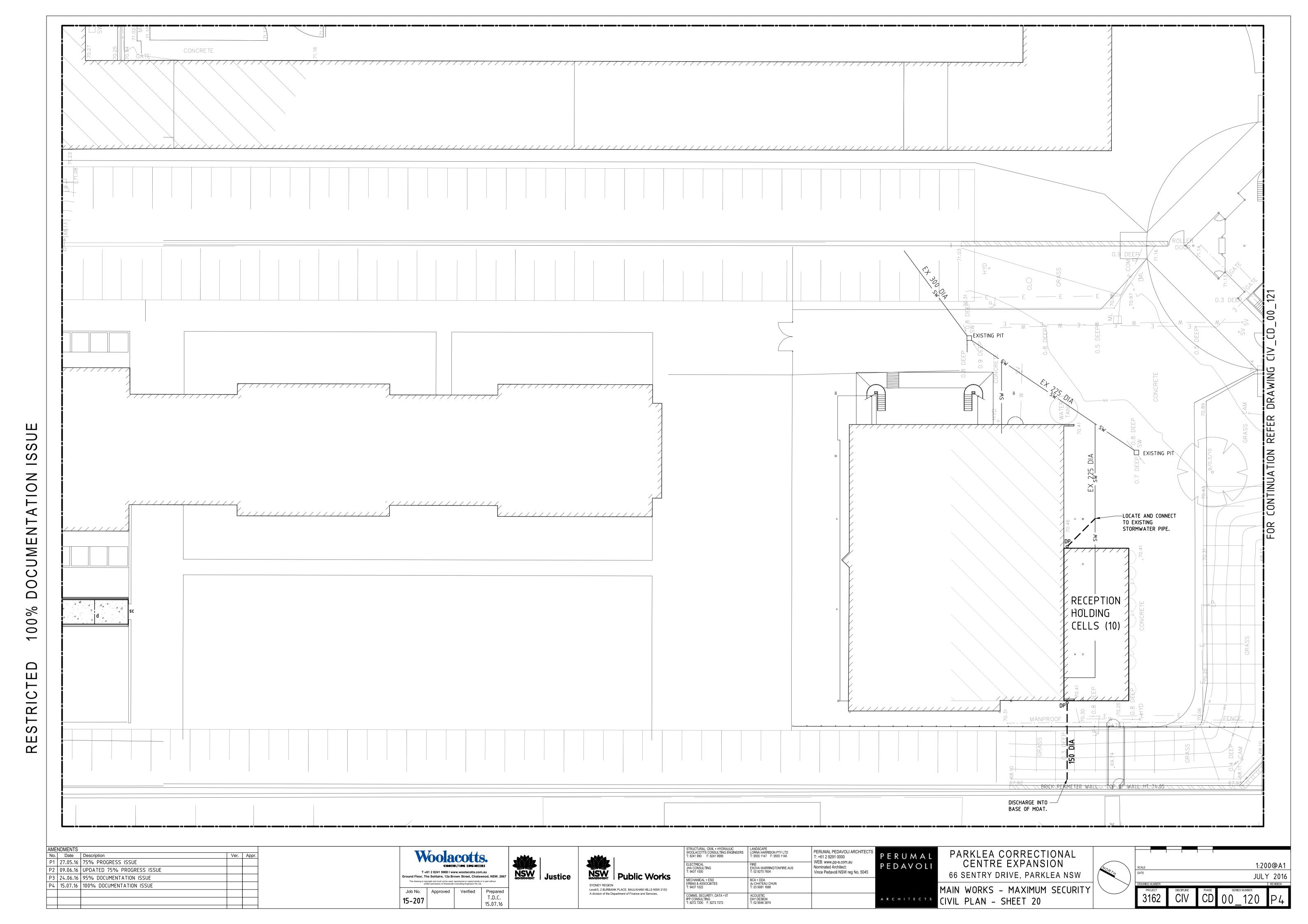


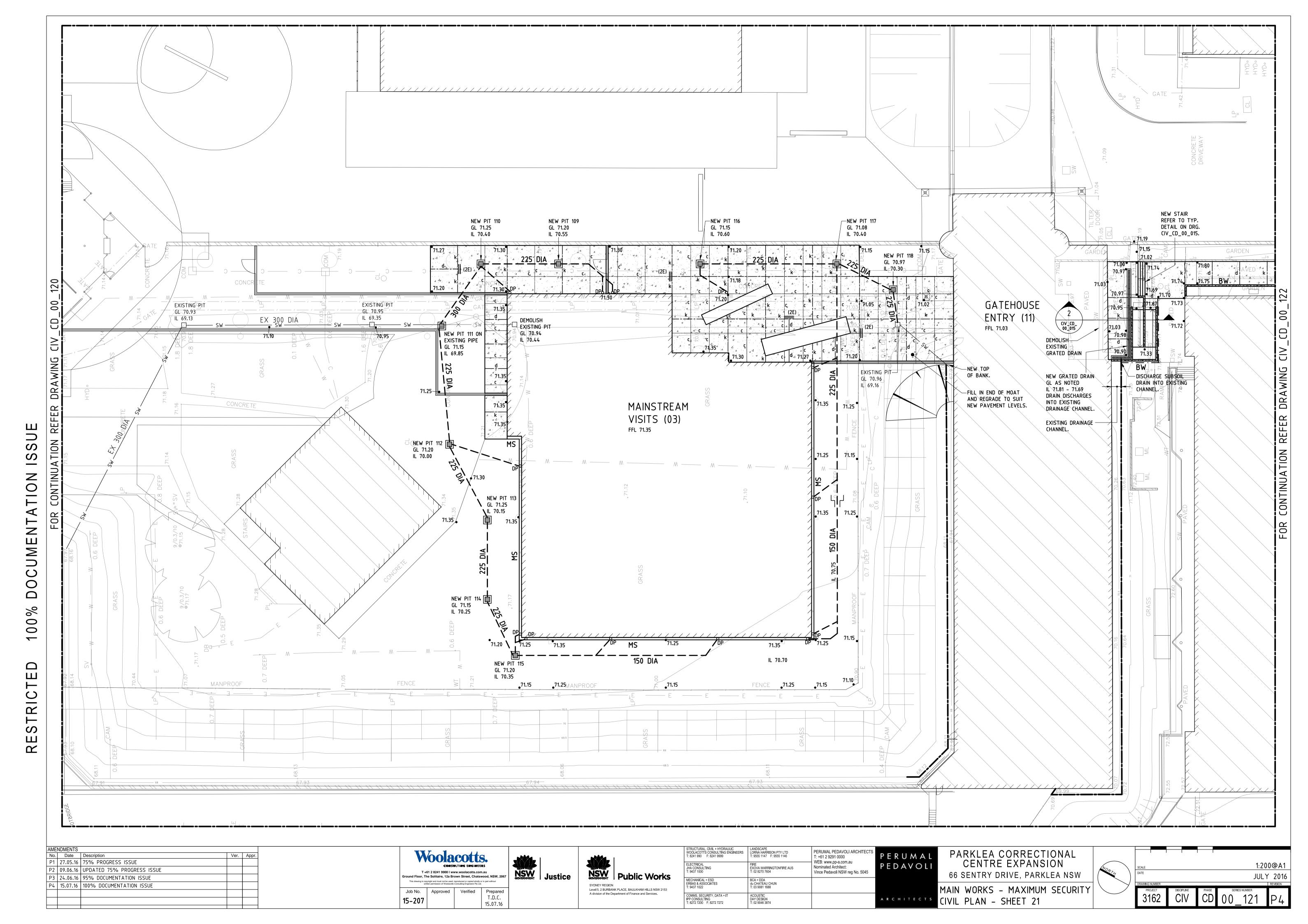


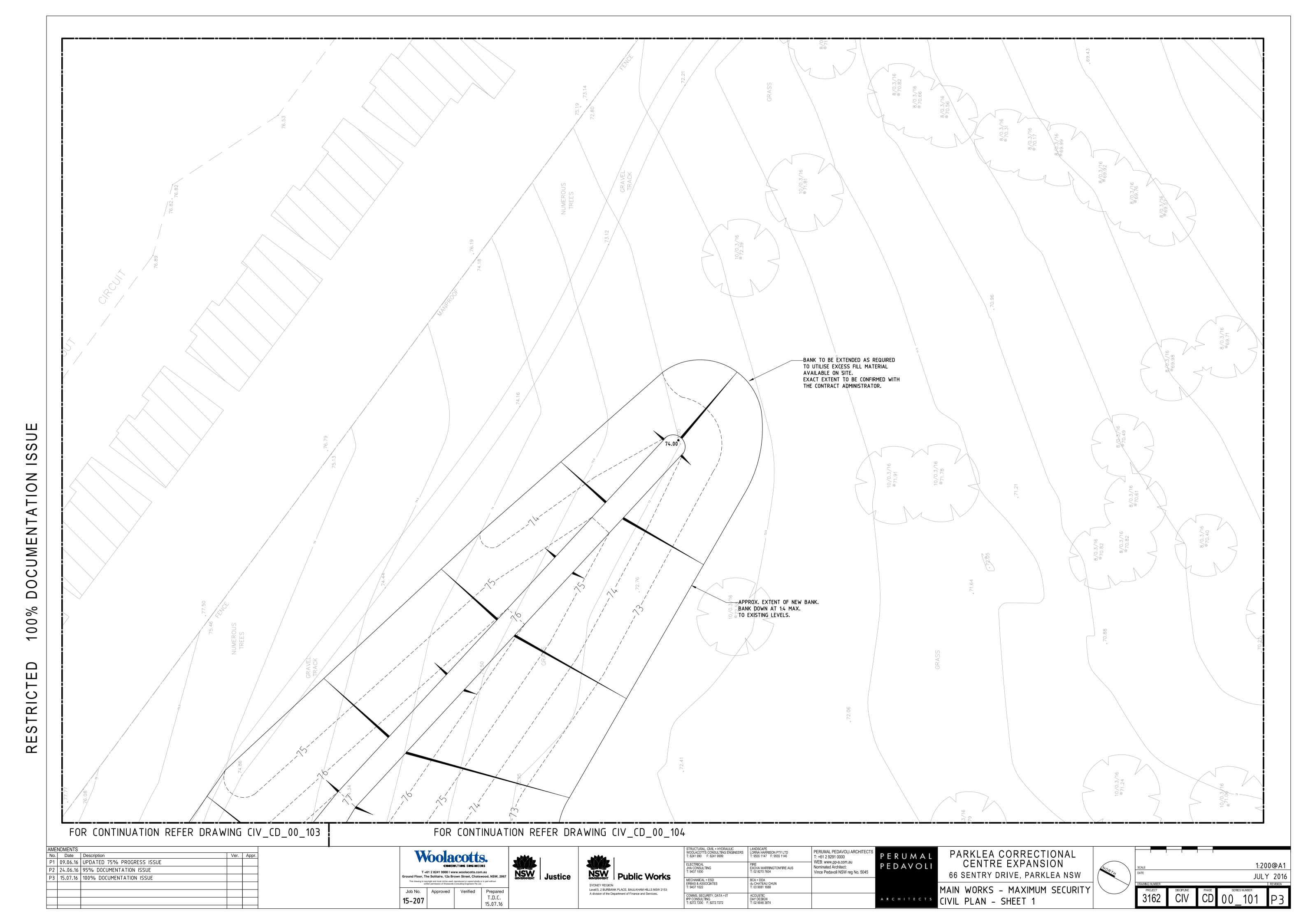


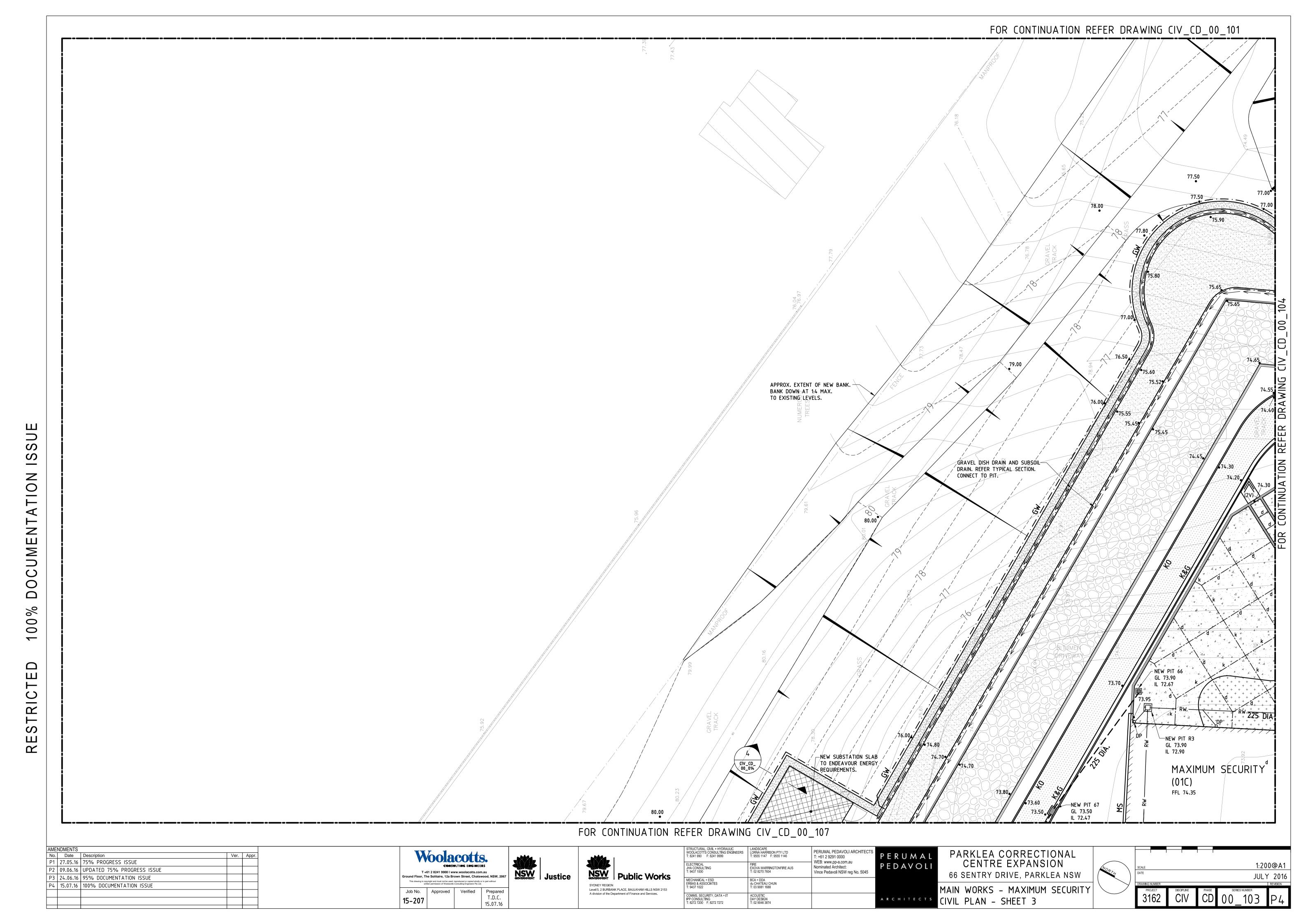












SYDNEY REGION Level 5, 2 BURBANK PLACE, BAULKHAM HILLS NSW 2153 A division of the Department of Finance and Services.

COMMS, SECURITY, DATA + IT IPP CONSULTING T: 8272 7200 F: 8272 7272

ACOUSTIC DAY DESIGN T: 02 9046 3874

MAINWORKS - MAXIMUM SECURITY

CIVIL PLAN - SHEET 36

ARCHITECTS

Appendix B Calculations

www.woolacotts.com.au

Designed ...



	PARKLEA		Date 20/1/16
*	DESILN	MEDRINATION SUMMARY	
	OSP	CAPAUTY DESIGN	
	PROJEC	T - Parkten expansion	
	REFER	ences = - AS35003 (2015) - Stormunter	Drainage
		- Australian Rainfall & Runoff (1018	2)
		- XP RAFTS - XPSOILHOUS Ma.	w.\.
		- Blancom Concil Regularments.	
	IFD =	Bom webs:te	
		Location: Lat = -33.725 South	
		40ng= 150-925 EAST.	
	050 =	- Post - develous outflow to pre-du	en works
		0/15.	
		- Modifican of Northern Dem (Som	miso of spilling

Job no. 15-207

Sheet no.

54

Designed



JOB PARKLEA Date 10/2/16 * XPRAFTS - Modification * Existing (A/11/15) FILE = PreDeveloped 2. XP * Post Developed FILE = Rost Deviloped 3-XP (12/11/15) To be modified. Spillway: soutedon = 69.333 North day = 68.05 (50 mon mised Spillum) - Existing = 68.00 Containent Areas (post developed) $\begin{array}{c} \text{New works only} = ... 796 + 207 + (79 \times 4) + 444 + 444 + 417 + 419 - \\ (\text{impervises}) + 1819 + 517 + (1894 \times 4) + 2314 + 1739 + \\ 2086 + 1163 + 756 + 2559 + 3107 + 2664 + ... \\ + (580 \times 3) + (444 \times 2) + 299 + 687 + 115 + 1217 \end{array}$ = 34 289 m2 1 - 2664 (som cerpra) 31625 m2 (100 Noth Dun) Soln: - Raise dan spilling by 50mm, will then reduce flows to less than existing Allows for as detention in upstream area, and for current impervous onens.

Job no. 15-207

Sheet no.

54

Designed

www.woolacotts.com.au



Parklea	Date	10/2/16
-> Post developed site		
nodification of existing model		
OUTH Dam		
enterment 2 (Internal)		
- 72 (2) 3		-
T = 72 676 m ²		-
1 impervious = 8748 + 2664		
- 11412 2		
Aimpervious = $8748 + 2664$ (Carpen 5040) = 11412m^2 A permus = 61264m^2		
510per = 2.5 th		
South Dan		
atument (External)		
Stormat (Landay)		
AT = 369110m2		
Aprimers = 115416m2		
2 2 3 494 2		
A Impervous = 263694 m ² Slope = 2-5 %		
31000-213		
North Dam		
Catchment 3 (Internal)		
transfer to the second		
AT = 428860 m		
AT = 428860 m ² April 3 = 233 308 A Imperiors = 163 927 + 31625 (pro hurlond) = 195552 m ² Slope = 1.6 %		
A TO 260 ciny = 163 927 + 31625		
(pro developer).		
= 195552212		
5lope = 1.6%		
Work Durn		
Carcumont 4 (EXX)		
AT = O (NOW force will be Constructed		
and get as a barrer to overland		
AT = 0 (New force with be Constructed one get as a borrow to overlead after)		

15-207

Sheet no. ...

54

Designed ...

www.woolacotts.com.au



b	irkleg	Da	te!	0/2	116
050	SOUTH - DAM.				
	- hvez = 67.5 (7WL)				
Ortlet	pipe = 50 mm = NA				
-,					
24000	e: LVL (m) Stor. (m3)				
	67-50 0 69-333 14349				
	70.00 19570				+
	70.00 ((5)				
Spi	Iwac lose = 69.333				
50.	Musy width = 20m				
050	NORTH - DAM.				
	- Invert = 67-50				
Outle	+ pipe > 600 mm				
5100	1 () (() (() () () () () () (
24018	62-50 0			+++	
	e: LVL (m) (stor (m)) 67.50 0 68.00 29406				
	68.50 58812				
Spillo	width = 60m				
Spille	width = 60m				
	9				
				-	
				-	-

15-207

Sheet no.

Job no.

54

Designed

www.woolacotts.com.au



	Level in	ARI	(n) too D	Level in	ARI Dam	Qout. (N)
	SOUTH	NORTH	(m3/s)	SOUTH	NORTH	(m ³ /s)
(0	68.40	67-58	0-26	68-76	67-62	0.34
20	68.93	67.64	0-37	69-38	67.71	0.46
30	69-31	67-68	0.42	69.53	67-82	0.57
45	69.46	67.77	0-52	69-64	67-95	0.68
12	69.53	67-85	0.59	69.67	68.05	0.76
1-54-	69.56	67.96	0.69	69-66	68.14	3.37
24	69-58	68.05	0.74	69.68	68.17	4.80
34	69-59	68.11	2.39	69-64	68.18	5.86
6h-	69.60	68-13	3.28	69.69	68.20	6.60
121	69-63	68.17	5.27	69-67	68.25	10.09
24 L-	69-56	68.18	5-86	69.60	68.23	8.67
48 L	69-53	68.19	2-86	69.57	68-21	7.48
1AX	69-63	68.19	5.86	69.69	68-25	10.09
XIST.	69.65	68/14	5.94	69.71	68.21	10-42

15-207

Sheet no.

Job no.



Sheet no.

Jolo	Parklen					Date 10/2/16
	* Pre-	Developea	Cesurs			
	2051 Level in 500 TH	- ARI Dum North	(m ² /\$)	Level 5	M Dam NORTH	Q out (N)
10	68.39	67-58	O-25	68-76	67.62	0.34
2.5	68.92	67.64	0-37	69.38	67.71	0-46
D	69.30	67.68	0.42	69-54	67.83	0-57
4-5	69.46	67.77	0-52	69-65	67.96	0.68
L	69.54	67-85	0.59	69-69	68.04-	1-51
1.51	69-58	67.96	0.69	69.68	68.11	4.26
2 L	69.60	68.04	1-37	69.70	68.13	5.56
7L-	69-60	68.08	2.97	69.67	68.12	6-30
6Lr	69.61	८४ - १०	3.66	69-71	68:16	7.28
121,	69-65	68.13	5.28	69-69	68.21	10-42
24L-	69.58	68.14	5.94	69.62	68.19	4.78
48Lr	69-55	68.14	5.93	69-58	68.17	7-57
MAX	69.65	68.14	5 94	69-71	68.21	10-42
Aprily, parameters,						
	<u> </u>					

34 Job no. 15-2-07

www.woolacotts.com.au



Job Parklea Date 28/10/15 XP-RAFTS Existing Size (pre-developed) MORTH DAM NORTH DAM CATCHMENT 4 (EXt.) CATCHMENT 3 (Int) AT = 13490 m2 A-T= 428860 m2 Apervious = 4874 m2 Apernos = 264933m2 Almpervious = 8616 m2 SOUTH DAM A Imperson = 163927 m2 510Pe= 1% CATCHMENT 2 (Internal) Slope = 1.6% AT = 72676 m >00 Apenions = 63928 m2 OSD SOUTH OSD NORTH OUTLET Almperusus = 8748 m Outlet Invert = 67.5 (TWO Outlet horest = 67-5 PRE-DEVELOPED Slope = 2.5 % Outlet pice - 50mm = NA Outlet pipe = 600mm STORAGE: LULIM) | Stor. (MB) STORAGE: LVL (m) Storms) SOUTH DAM 67.50 65,00 CATCHMENT (EXHCM) 69.333 14349 67.50 0 AT= 369110 m 70.00 19570 68.00 29406 Spillway level = 69.333 Apernus = 115416 m2 68.50 58812 A impervious = 253694 m2 Spillway width= 18m Sp. 1100 1 evel = 68.00 Slogn = 25%. Spilling with = 58 Discharge : Staged discharge * MODEL DATA: configuration - Initial/continuing Losses Mannings: (Pervious) N = 0.025 Pervious = 10 mm/hr Initial N= 0.015 (Impernous) 2.5 mm/hr continuous Imperiors = 2 mm/hr Initial O mm/hr continuous IFD Gefficients: Bom (parklen) Lat = -33.725 South f2 = 4-3 Skew= 0.01 Long = 150.925 East fs= 15-82 2yr 1hr = 31.1 mm/hr Sourth = 59.9 mm/hr 2 y 12 hr = 6.95 mm/hr 50 gr 124= 13.8 mm/hr Zyr 72hr= 2.02 mm/hr 50y- 724, = 4,65 my/. 54 Job no. 15-207



